# This electronic collection of documents is provided for the convenience of the user and is Not a Certified Document –

The documents contained herein were originally issued and sealed by the individuals whose names and license numbers appear on each page, on the dates appearing with their signature on that page.

This file or an individual page shall not be considered a certified document.

STATE PROJECT REFERENCE NO. SHEETS B-572219

# STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

# **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY ROCKINGHAM

PROJECT DESCRIPTION BRIDGE NO. 277 ON SR 1169 (ISLAND DRIVE) OVER BIG BEAVER ISLAND CREEK

## **CONTENTS**

REFERENCE: 45678.

SHEET NO.	<b>DESCRIPTION</b>
1	TITLE SHEET
2, 2A	LEGEND (SOIL & ROCK)
2B, 2C	SUPPLEMENTAL LEGEND (GSI)
3	SITE PLAN
4-13	BORE LOGS & CORE REPORTS
14-15	CORE PHOTOGRAPHS
16	SITE PHOTOGRAPHS

PERSONNEL

**TRIGON** 

GOODNIGHT, D.G.

KLEINFELDER

INVESTIGATED BY GOODNIGHT, D.G.

DRAWN BY \_\_CROCKETT, S.C.

CHECKED BY <u>HAMM</u>, J.R.

SUBMITTED BY \_FALCON ENG.

DATE MARCH 2022

# **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1(99) 707-850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

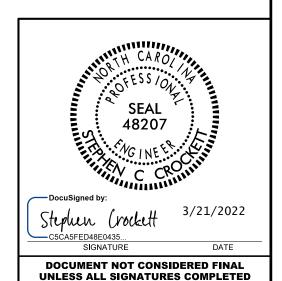
CENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (INP-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOL THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION, THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEM NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED TO THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

  I. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

  BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY MAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.



PROJECT REPERENCE NO. SHEET NO. 2

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

# SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS (PAGE 1 OF 2)

SOL DESCRIPTION  SOL DE												(PA	4GE	I OF 2)				
Sent Is concerned to Accordance for Control Sent Concerned from the American Control C						SOI	L DE	SCR	[PT]	ON				GRADATION				
The content of the	BE PENE ACCORD IS	TRATED W DING TO TH BASED ON	ITH A ( HE STAN THE A	CONTII NDARD ASHTO	NUOUS PENET SYSTE	D, SEMI FLIGHT FRATION EM. BA	-CONSO F POWEI N TEST SIC DE:	ILIDATE R AUGE (AASH SCRIPT	ED, OR ER AND ITO T IONS (	WEATHERED YIELD LES 206, ASTM GENERALLY	SS THAN 101 D1586). SOII INCLUDE TH	Ø BLOWS PI L CLASSIFI HE FOLLOWI	ER FOOT CATION NG:	UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.  GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.				
MINING   1975	AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,																	
Company   Comp	VERY STIFF,GRAY,SILTY CLAY,MOIST WITH INTERBEDDED FINE SAND LAYERS,HIGHLY PLASTIC,A-7-6																	
MAIL PROPRIEST   1	CENERAL CRANIII AR MATERIALS SILT-CLAY MATERIALS											MINERALOGICAL COMPOSITION						
SAME   1	CLASS. GROUP	A-1	(≤ 35	PASS	ING #200	8) 4-2		( > 3	5% PAS	SING #200) A-6 A-7	A-1, A-2	A-4. A-5	IALS	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.				
RESIDE				A-2-	4 A-2-5			\$501855501		A-7-5. A-7-6	A-3	A-6, A-7	***************************************					
Texture   Constitute   Consti		00000000	80			23			171					MODERATELY COMPRESSIBLE LL = 31 - 50				
10   10   10   10   10   10   10   10		50 MX									GRANULAR		MUCK,					
MORNEY		30 MX 50 I			1X 35 MX	x 35 MX	35 MX	36 MN	36 MN	36 MN 36 MN			PEAT	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS OTHER MATERIAL				
The content of the												1		TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%				
## 0.		-	_	40 M	1X 41 MN	40 M	41 MN	40 MX	41 MN	40 MX 41 MN				MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%				
Marting   Section   Sect				10 M	10 MX	+	_		_		MODI	ERATE						
STATE WATER LEYE, AFTER 24 HOURS   STATE WATER LEYER, AFTER 24 HOURS   STATE WATER LEVER, AFTER 24			_		0	4	MX	8 MX	12 MX	16 MX NO MX								
PRIVE   DESCRIPTION   DESCRI	OF MAJOR	GRAVEL, AN	u   FINE															
### SAMONE SERVED  **CONSISTENCY OF DENSENSS**  **CONSISTENCY OF SHORT OF DENSENSS**  **CONSISTENCY OF			- FYCEI	LENT	TO COOD				FAID TO	n noon	FAIR TO	DOOD	UNCULTADI E					
COMBISTENCY OF DENSINESS   STEP   COMPACINESS   STEP   STORE   STORE   STEP   STORE	AS SUBGRADE											PUUR	UNSUITABLE	O-MM- SPRING OR SEEP				
PRIMARY SOIL TYPE			PI OF											-				
MEDIUM GENEE   19 10 30   10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 30   10 10 30   10 10 30   10 30   10 30   10 30   10 30   10 30   10 30   10 30   10 30   10 30   10 30   10 30   10 30   10 30	PRIMARY	SOIL TYP	E						RATION	RESISTENCE		PRESSIVE S	TRENGTH	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION				
MEDIUM GENEE   19 10 30   10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 10 30   10 30   10 10 30   10 10 30   10 30   10 30   10 30   10 30   10 30   10 30   10 30   10 30   10 30   10 30   10 30   10 30   10 30	GENERA	ALLY		٧E										SOIL SYMBOL SPT DOT TEST BORING SLOPE INDICATOR				
VERY SIFF   10 10   10   10   10   10   10   10	GRANUL	GRANULAR LUUSE 4 10 10  MATERIAL MEDIUM DENSE 10 TO 30 N/A					30		N/A		¥ 151 FM1 = 1.10							
CREARILY   MODITURE   2 TO 4   8.25 TO 8.5   8.25 TO 1.5   1 TO 2   1 TO	(NON-CI	DHESIVE)			RY DE	NSE			> !	50								
MATERIAL   NATIONAL	GENERA	ALLY		V										Y				
VERY STIFE   15 TO 30				ME										WITH CORE				
TEXTURE   OR GRAIN SIZE	(COHES	IVE)		VE									4					
DORENING IMM0							RE O	R GF			I			RECOMMENDATION SYMBOLS				
BOULGER COBBLE (GRAVEL COARSE FIRE SAND (CSE.SO.) (FS.D.) (FS.D.) (SL.) (CL.)  BOULGER (COB.) (GR.) (CSE.SO.) (FS.D.) (FS.D.) (SL.) (CL.)  GRAIN MM 305 75 2.0 0.25 0.05 0.005  SIZE IN. 12 3 3 0.05 0.005  SOIL MOISTURE - CORRELATION OF TERMS  SOIL MOISTURE - CORRELATION OF TERMS  SOIL MOISTURE SCALE (ATTERBERG LIMITS) (CSE.SO.) (CSE.SO																		
CRAIN MM 385 75   2.8   0.25   0.85   0.805	BOULDE	ER			GRAV	VEL	2.00	COARS	SE.	FIN	E D			SHALLOW UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF				
SOLE MOISTURE - CORRELATION OF TERMS  SOLE MOISTURE SCALE  SOLE MOISTURE SCALE  FIELD MOISTURE DESCRIPTION  OUIDE FOR FIELD MOISTURE DESCRIPTION  COUIDE FOR FIELD MOISTURE DESCRIPTION  OUIDE FOR FIELD MOISTURE DESCRIPTION  COMPONENT FROM BELOW THE GROUND WATER TABLE  PLASTIC  (ATTERBERG LIMIT)  PLASTIC  PLASTIC  (PI)  OM OPTIMUM MOISTURE  SL SHINKAGE LIMIT  OM OPTIMUM MOISTURE  SL SHINKAGE LIMIT  NON PLASTIC  OM OPTIMUM MOISTURE  SL SHINKAGE LIMIT  NON PLASTIC  OM OPTIMUM MOISTURE  SL SHINKAGE LIMIT  NON PLASTIC  OM OPTIMUM MOISTURE  SL SHINKAGE LIMIT  OM OPTIMUM MOISTURE  OM OPTIMUM MOISTURE  SL SHINKAGE LIMIT  OM OPTIMUM MOISTURE  OM OPTIMUM MOISTURE  SL SHINKAGE LIMIT  OM OPTIMUM MOISTURE  OM OPTIMUM MOISTURE  OM OPTIMUM MOISTURE  SL SHINKAGE LIMIT  OM OPTIMUM MOISTURE  OM OPTIMU			(COB.)		lur			CSE. S			0.)							
SOIL MOISTURE SCALE SOIL MOISTURE SCALE (ATTERBERG LIMITS)  SOIL MOISTURE SCALE (BILD MOISTURE SAPL SABREVIATIONS SAP. SAPROLITIC SAMPLE ABBREVIATIONS SAP. SAPROLITIC SAPPLE ABBREVIATIONS SAPLE ABBREVIATIONS SAPROLITE SAPPLE ABBREVIATIONS SAPROLITE SAPPLE ABBR							2.0		(	0.25	0.05	0.005	5					
SOIL MOISTURE SCALE (ATTERBERG LIMITS)  COURT FIELD MOISTURE DESCRIPTION  GUIDE FOR FIELD MOISTURE DESCRIPTION  DESCRIPTION  GUIDE FOR FIELD MOISTURE DESCRIPTION  DESCRIPTION  COST. ORGANIC  PHO PRESSUREMETER TEST DOT - DYNAMIC PENETRATION TEST SAP. SAPROLLITIC  SOD. SAND, SANDY SS - SPLIT SPOON SS - SPLIT SPOO			SOTI	М	OIST	URF	- C(	ORRE	ΙΑΤ	ION OF	TERMS							
CATTERBERO LIMITS  DESCRIPTION			E SCAL	.E		FIEL	D MOIS	STURE				STURE DES	SCRIPTION	CSE COARSE ORG ORGANIC				
CSAT_J   FROM BELOW THE GROUND WATER TABLE   F - FINE   SL SILT, SILTY   ST - SHELBY TUBE   RS - ROCK   FRACTURES   FRACE - FRACTURED, FRACTURED, FRACTURES   FRACE - FRACTURED, FRACTURES   FRACTURED, FRACTURES   FRACE - FRACTURED, FRACTURES   FRACE - FRACTURED, FRACTURES   FRACE - FRACTURED, FRACTURES   FRACTURED, FRACTURED   FRACTURED, FRACTURES   FRACTURED, FRACTURED	(AT	TERBERG	LIMITS	)														
LL LIOUID LIMIT PLASTIC RANCE (PI) PL PLASTIC LIMIT  OM OPTIMUM MOISTURE SHRINKAGE LIMIT  OM OPTIMUM MOISTURE SHRINKAGE LIMIT  ON PLASTIC LIMIT  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  SULIFORM MOISTURE  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  SULIFORM MOISTURE  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  SHRINKAGE LIMIT  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  - MOIST - (M) SOLID; FILIONE SELIFORNIA BEARING RATIO  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  - MOISTURE SELIFORNIA BEARING RATIO  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  - MOISTURE SELIFORNIA BEARING RATIO  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  - MOISTURE SELIFORNIA BEARING RATIO  - MOIST - (M) SOLID; FILIONE SELIFORNIA BEARING RATIO  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  - MOISTURE SELIFORNIA BEARING RATIO  - MOISTURE SELIFORNIA BEARING  RATIO  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  - MOISTURE SELIFORNIA BEARING  - MOISTURE SELIFORNIA BEARING  - MOISTURE SELIFORNIA SILI, SLIGHTLY V - VERY  - MOISTURE SELIFORNIA BEARING  - MOISTURE SELIFORNIA BEARING  - MOISTURE SELIFORNIA SILI, SLIGHTLY V - VERY  - MOISTURE SELIFORNIA  - MOISTURE SELIFORNIA SILI, SLIGHTLY V - VERY  - MOISTURE SELIFORNIA SILI, SLIGHTLY V								ED -										
RANGE (PI) PL PLASTIC LIMIT		. <del> </del> LIQU	ID LIM	T	-									FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK				
OPTIMUM MOISTURE SHRINKAGE LIMIT  OPTIMUM MOISTURE SHRINKAGE LIMIT  OPTIMUM MOISTURE SHRINKAGE LIMIT  OPTIMUM MOISTURE SHRINKAGE LIMIT  OR Y - (D)  REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  OR - 55  OR - 45C  OR - 55  OR + OLD WADGERS  OR + OLD WADGERS  OR + OLD WADGERS  OR - STEEL TEETH  HAND AUGER  HIGH  PORTABLE HOIST  WADBILE B-57  WAN BILE B-57  OR SUBJECT  PROJECT  DRILL UNITS:  ADVANCING TOOLS:  CHC - 45C  CALY BITS  OR - 45C  OR - 55  OR -	RANGE 2	D. A.C.	TIC . 1			- WE	T - (W							FRAGS FRAGMENTS $w$ - MOISTURE CONTENT CBR - CALIFORNIA BEARING				
ORILL UNITS: SHRINKAGE LIMIT  - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  - DRY - (D) REQUIRES ADDITIONAL WATER TO CASE SITE IN ANY ADMANCER  - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  - DRY - (D) REQUIRES ADDITIONAL WATER TO CASE SITE IN ANY ADMANCER  - DRY - (D) REQUIRES ADDITIONAL WATER TO CASE SITE AND ANY ADMANCER  - DRY - (D) REQUIRES ADDITIONAL WATER TO CASE SITE AND AND AUGER  - DRY - (D) REQUIRES ADDITIONAL WATER TO CASE SITE AND AUGER  - DRY - (D) REQUIRES ADDITIONAL WATER TO CASE SITE AND AUGER  - DRY - (D) REQUIRES ADDITIONAL WATER TO CASE SITE AND AUGER  - DRY - (D) REQUIRES ADDITIONAL WATER TO CASE SITE AND AUGER  - DRY - (D) REQUIRES ADDITIONAL WATER TO CASE SITE AND AUGER  - DRY - (D) REQUIRES ADDITIONAL WATER TO CASE SITE AND AUGER  - DRY - (D) REQUIRES ADDITIONAL WATER TO CASE SITE AND AUGER  - DRY - CAS BITS  - DRY - (CAS BITS  - DRY - (CHC - 550  - DRY	PL L	+ PLAS	STIC LI	MII										T 12 12 12 12 12 12 12 12 12 12 12 12 12				
CME-45C CLAY BITS    PLASTICITY  PLASTICITY   DRY STRENGTH   CME-550   CME-5	40 12	OM _ OPTIMUM			IRE r	- ML	1151 -	(M)		SULID; AI	JR NEAR U	PIIMUM MU	JISTURE	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:				
PLASTICITY  PLASTICITY INDEX (PI)  NON PLASTIC  SLIGHTLY PLASTIC  SLIGHTLY PLASTIC  G-5  VERY LOW SLIGHTLY PLASTIC  HIGHLY PLASTIC  16-25  MEDIUM HIGHLY PLASTIC  COLOR  DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).  CME-55Ø  HARD FACED FINGER BITS  TUNG,-CARBIDE INSERTS  X -N O  TUNG,-CARBIDE INSERTS  X -N O  TUNG,-CARBIDE INSERTS  X -N O  TUNG,-CARBIDE INSERTS  TRICONE  TRICONE  TRICONE  TRICONE  TRICONE  TRICONE  TRICONE  TUNG,-CARB.  SOUNDING ROD  VANE SHEAR TEST		Ţ ,,,,				- DR	Y - (D)	)					0	6 CONTINUOUS FLIGHT AUGER CORE 5175.				
PLASTICITY INDEX (PI)  NON PLASTIC SLIGHTLY PLASTIC SLIGHTLY PLASTIC HIGHLY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH  DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).  PLAST STRENGTH VERY LOW SLIGHT VANE SHEAR TEST  CME-55Ø  HARD FACED FINGER BITS  TUNGCARBIDE INSERTS  X -N Q  TUNGCARBIDE INSERTS  WANDSHEE HOIST TRICONE TRICONE TRICONE TRICONE TRICONE TRICONE TRICONE TUNGCARB. SOUNDING ROD VANE SHEAR TEST								דורי				CME-55						
NON PLASTIC 0-5 VERY LOW SLIGHTY PLASTIC 6-15 SLIGHT MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER  COLOR  DESCRIPTIONS MAY INCLUDE COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).										>1)	n	CMC 559						
MODERATELY PLASTIC 16-25 MEDIUM HIGHY PLASTIC 26 OR MORE HIGH PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER  COLOR  DESCRIPTIONS MAY INCLUDE COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).		NON PLASTIC 0-5 VERY LOW							(	<u></u>	<u> </u>	TUNGCARBIDE INSERTS						
PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER  COLOR  DESCRIPTIONS MAY INCLUDE COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	MO	DERATELY	PLAST	IC				16-25				MEDIUM		X CASING W/ ADVANCER				
COLOR  DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).    CORE BIT   VANGCARB.   SOUNDING ROD   VANE SHEAR TEST	HIO	HLY PLAS	STIC									HIGH		PORTARIE HOIST TRICONE STEEL TEETH				
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).							CC	DLOR						TRICONE TUNGCARB. SQUADING POD				
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.														X CORE BIT VANE SHEAR TEST				
	M	ODIFIERS	SUCH A	as LI	GHT, DA	ARK.S	TREAKE	D, ETC	. ARE	USED TO I	DESCRIBE 4	APPEARANCI	E					

PROJECT REFERENCE NO SHEET NO B-57222Α

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

# SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

	SOIL MED ROOK ELGEND, ILMING
	(PAGE 2
	ROCK DESCRIPTION
ROCK LINE II SPT REFUSAL BLOWS IN NO REPRESENTED	IS NON-COASTAL PLAIN MATERIAL THAT MOULD VIELD SPT REFUSAL IF TESTED. AN INFERRED NDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. LIS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 ON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN D BY A ZONG OF WEATHERED ROCK.  1ALS ARE TYPICALLY DIVIDED AS FOLLOWS:
WEATHERED ROCK (WR)	NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.
CRYSTALLINE ROCK (CR)	GNEISS, GABBRO, SCHIST, ETC.
NON-CRYSTAL ROCK (NCR)	SEDIMENTARY RUCK THAT WOULD YELD SPI REFUSAL IF TESTED.  ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.
COASTAL PLA SEDIMENTARY (CP)	Y ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.
	WEATHERING
FRESH	ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.
VERY SLIGHT (V SLI.)	ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.
SLIGHT (SLI.)	ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO I INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.
MODERATE (MOD.)	SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.
MODERATELY SEVERE (MOD. SEV.)	ALL ROCK EXCEPT OUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTI AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES 'CLUNK' SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL
SEVERE (SEV.)	ALL ROCK EXCEPT OUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.  IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF
VERY SEVERE (V SEV.)	ALL ROCK EXCEPT OUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTICES OF ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, MOULD YIELD SPT N VALUES &lt; 100 BPF</i>
COMPLETE	ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.
	ROCK HARDNESS
VERY HARD	CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.
HARD	CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.
MODERATELY HARD	CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.
MEDIUM HARD	CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.
SOFT	CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.

FRACTURI	E SPACING	BEDD1	NG
TERM	SPACING	<u>TERM</u>	THICKNESS
VERY WIDE	MORE THAN 10 FEET	VERY THICKLY BEDDED	4 FEET
WIDE	3 TO 10 FEET	THICKLY BEDDED	1.5 - 4 FEET
MODERATELY CLOSE	1 TO 3 FEET	THINLY BEDDED	0.16 - 1.5 FEET
CLOSE	Ø.16 TO 1 FOOT	VERY THINLY BEDDED	0.03 - 0.16 FEET
VERY CLOSE	LESS THAN 0.16 FEET	THICKLY LAMINATED	0.008 - 0.03 FEET
		THINLY LAMINATED	< 0.008 FEET

CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNALL.

### INDURATION

VERY

SOFT

FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. RUBBING WITH FINGER EREES NUMEROUS GRAINS. GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE: MODERATELY INDURATED BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE: INDURATED DIFFICULT TO BREAK WITH HAMMER. SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE: EXTREMELY INDURATED SAMPLE BREAKS ACROSS GRAINS.

### TERMS AND DEFINITIONS

JVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. IFER - A WATER BEARING FORMATION OR STRATA.

NACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING DTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.

ESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT CH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND

CAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. \_UVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM

E RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT KS OR CUTS MASSIVE ROCK.

- THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE

<u>DIRECTION (DIP AZIMUTH)</u> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE TOP DIP, MEASURED CLOCKWISE FROM NORTH.

- A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE S RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.

SILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.

 $rac{\Delta T}{2}$  - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIG<sub>I</sub>NAL POSITION AND DISLODGED FROM ENT MATERIAL.

OD PLAIN (FP) - LAND BORDERING A STREAM. BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. MATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE

IT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.

<u>GE</u> - A SHELF-LIKE LATERAL EXTENT. - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO

- A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.

TLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS ALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.

CHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE AN INTERVENING IMPERVIOUS STRATUM.

IDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.

COUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE AND EXPRESSED AS A PERCENTAGE.

ROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT

\_ - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND ATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.

- POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT SLIP PLANE.

NDARD PENETRATION TEST (PENETRATION RESISTANCE)(SPT) - NUMBER OF BLOWS (N OR BPF)OF 10 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL 1 A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL OR LESS THAN Ø.1 FOOT PER 60 BLOWS.

ATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY AL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.

<u>STRATA ROCK QUALITY DESIGNATION (SROD)</u> - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.

TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

BENCH MARK: BL-4 (968,387 FT N, 1,709,778 FT E)

ELEVATION: 611.79 FEET

FIAD - FILLED IMMEDIATELY AFTER DRILLING

TOP OF BRIDGE RAIL ELEVATIONS EBIDOWNSTREAM - 614.7 FT EB2 DOWNSTREAM - 614.6 FT

DATE: 8-15-14

PROJECT REPERENCE NO. SHEET NO. 2B

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

# SUBSURFACE INVESTIGATION

SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS (PAGE 1 OF 2)

FROM AASHTO LRFD BRIDGE  AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Join	DES	IGN SPE	CIFICATI	ONS (PAC		
GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)  From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.  STRUCTURE	SURFACE CONDITIONS	VERY GOOD Very rough, fresh unweathered surfaces	XX COOD Z Rough, slightly weathered, iron stained C surfaces	AS FAIR D Smooth, moderately weathered and altered surfaces	T POOR  Slickensided, highly weathered surfaces  with compact coatings or fillings  or angular fragments	VERY POOR     Slickensided, highly weathered surfaces     with soft clay coatings or fillings
INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities	CES	90			, N/A	N/A
BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets	- ROCK PIECES		70 60			
VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets	  RLOCKING OF			50		
BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity	INTE			40	30	
DISINTEGRATED - poorly inter- locked, heavily broken rock mass with mixture of angular and rounded rock pieces	DECREASING				20	
LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes	- W -	N/A	N/A			10

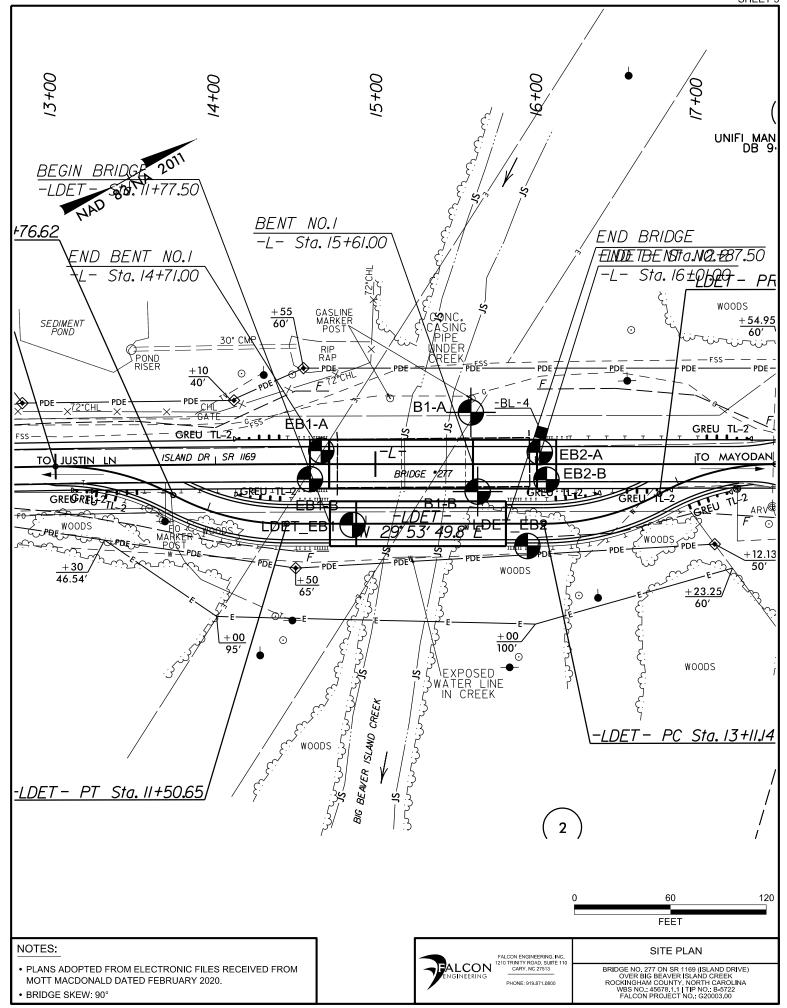
PROJECT REFERENCE NO. SHEET NO. 2C

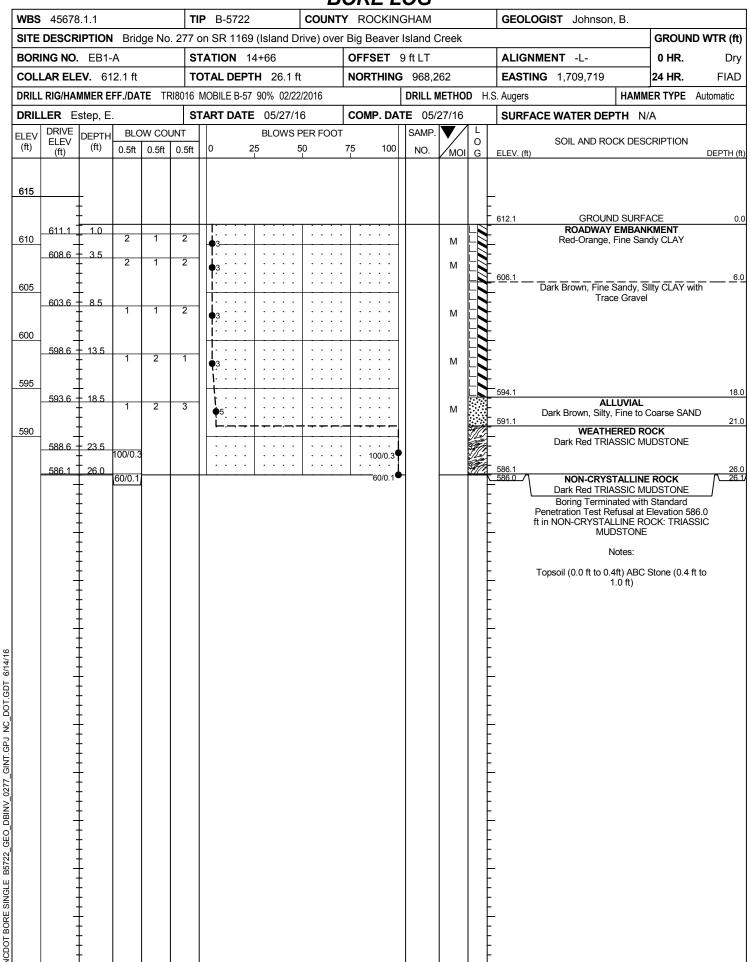
# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

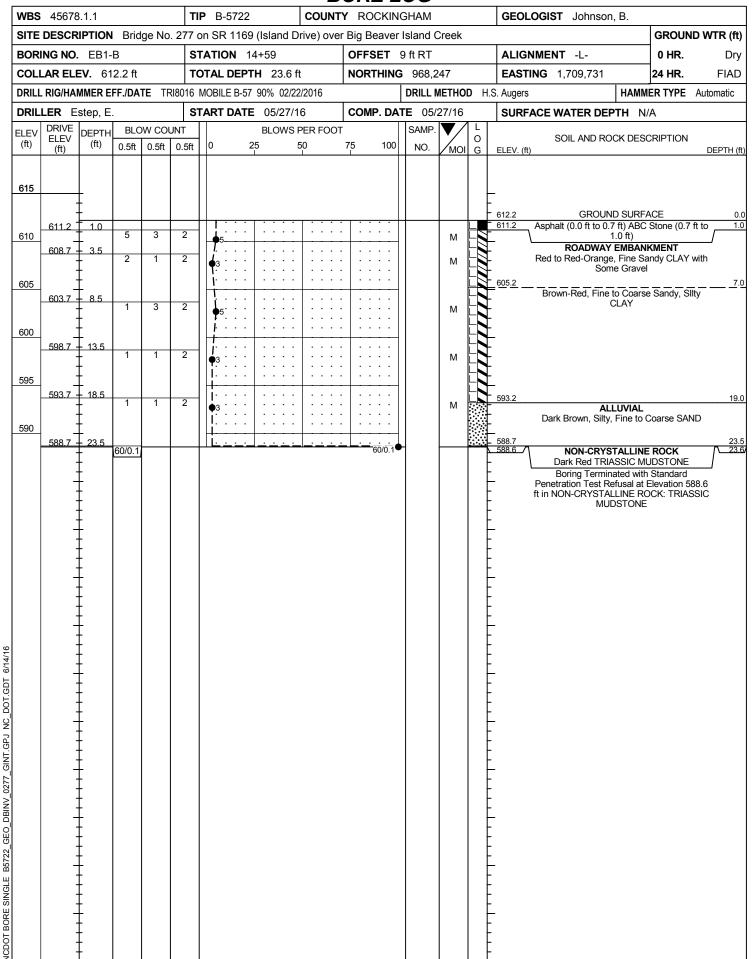
# SUBSURFACE INVESTIGATION

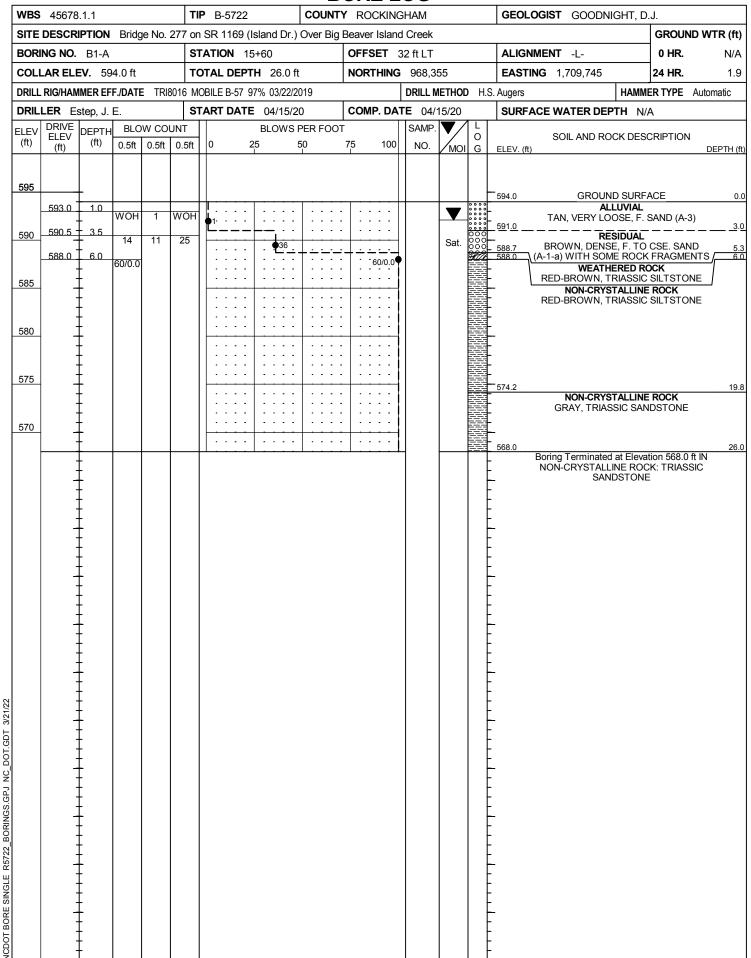
SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS (PAGE 2 OF 2)

FROM AASHTO LRFD BRIDGE DESIGN  AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Def					
GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos.P and Hoek E., 2000)					
From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.	VERY GOOD - Very Rough, fresh unweathered surfaces	GOOD - Rough, slightly weathered surfaces	FAIR - Smooth, moderately weathered and altered surfaces	POOR - Very smooth, occasionally slickensided surfaces with compact coatings or fillings with angular fragments	VERY POOR - Very smooth, slicken- sided or highly weathered surfaces with soft clay coatings or fillings
COMPOSITION AND STRUCTURE				, ,	, ,
A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.	70 60	A			
B. Sand- stone with stone and shin inter- layers of siltstone amounts  D. Siltstone or silty shale with sand- stone layers stone layers layers		50 B 40	C [	E E	
C.D.E. and G - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to F and H.			30	F/ 20	
G. Undisturbed silty or clayey shale formed silty or clayey shale forming a or without a few very thin sandstone layers of clay. Thin layers of sandstone are transformed into small rock pieces.			<b>\$</b>	/ 	10
─────────────────────────────────────					DATE: 8-19-16

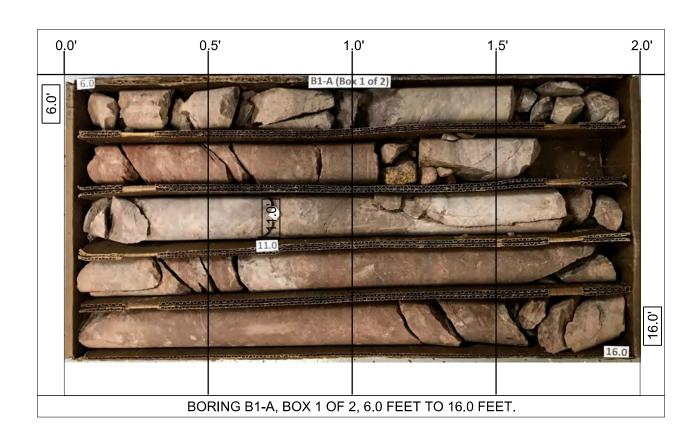


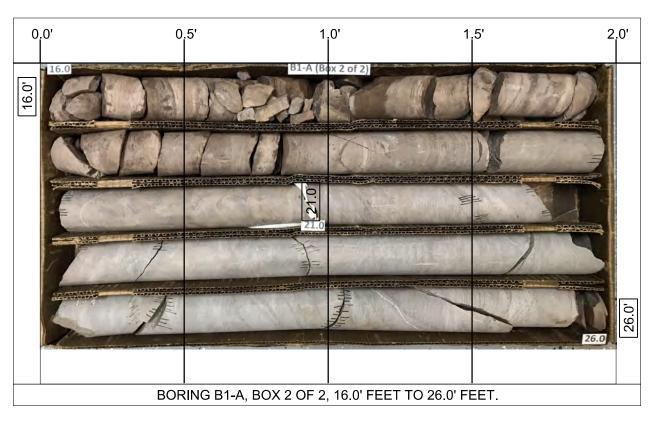


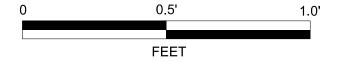




WPS	45678	2 1 1			TIP	B-572	22				RE LOG ROCKINGHAM GEOLOG		CHT D	1	
			I D-i-	lao No. O								GOODNI			D WATE (#
				ige N0. 2				ט טר.)	over I	<del>-</del>	eaver Island Creek	ENT '			D WTR (ft
	NG NO.				_		15+60	0 #		-	FSET 32 ft LT ALIGNME			0 HR.	N/A
	AR ELI				<u> </u>		<b>PTH</b> 26.		10	INC		1,709,745		24 HR.	1.9
				IE IRIOU					19	100	DRILL METHOD H.S. Augers	- WATER RED			Automatic
	LER E		E.				TE 04/1			CC	MP. DATE 04/15/20 SURFAC	E WATER DEP	IH N/A	\	
	RUN			DRILL		INI	<b>N</b> 20.0 f	STR	ATA	-					
(ft)	ELEV (ft)	DEPTH (ft)	RUN (ft)	RATE (Min/ft)	REC. (ft) %	RQD (ft) %	SAMP. NO.	REC. (ft) %	RQD (ft) %	Ö G	DESCRIPTION DESCRIPTION	N AND REMARKS	3		DEPTH (
588	588.0	6.0	5.0	6:15/1.0	(4.2)	(1.0)		(12.8)	(4.8)			oring @ 6.0 ft STALLINE ROCK			6
585	583.0	11.0	0.0	3:23/1.0 9:47/1.0 4:01/1.0 6:30/1.0	84%	20%		93%	35%		RED-BROWN, SLIGHTLY T MODERATELY HARD TO HARD FRACTURED, TRIASSIC	O VERY SLIGHTL , MODERATELY	LY WEATI	O CLOSE	
580	-	11.0	5.0	4:33/1.0 4:47/1.0 5:01/1.0	(4.9) 98%	(3.0) 60%					- - -				
	578.0	16.0	5.0	6:30/1.0 7:03/1.0 3:53/1.0 4:06/1.0	(4.9) 98%	(2.0) 40%					- - -				
575	-	Ŧ		3:59/1.0 4:35/1.0	3070	7070					-  574.2				19
570	573.0	21.0	5.0	5:55/1.0 4:45/1.0 4:56/1.0 6:25/1.0		(4.9) 98%		(6.2) 100%	(6.1) 98%		GRAY, VERY SLIGHTLY WE MODERATELY CLOSE TO 0		TO VER		
370	568.0	26.0		4:41/1.0							_	(STOINEVILLE FI	IVI.)		26
	300.0	20.0		5:05/1.0							Boring Terminated at Elevation 5TRIASSI	568.0 ft IN NON-C C SANDSTONE	RYSTALL	INE ROC	
	-	<u> </u>									- - -	0 0/ 11/20 1 01 12			
	-	‡									- -				
	-	‡									- <del>-</del>				
	-	‡									<del>-</del> -				
	-	‡									- -				
	-	Ī									- - -				
		Ī									- - -				
	-	‡									<del>-</del> -				
	-	‡									- -				
	-	‡									<del>-</del> <del>-</del>				
	-	‡									<u>-</u> -				
	-	Ŧ									<del>-</del>				
	-	Ŧ									-				
	-	Ŧ									- - -				
	-	1									-  -				
	-	ŧ									<u>-</u> -				
	-	ŧ									- <del>-</del>				
		Ī													
	-	ŧ									<u>-</u> -				
	-	‡									_ -				
	-	‡									- -				
	-	‡									- <del>-</del>				
		‡									- -				
	-	‡									<del>-</del> -				
	-	‡									<del>-</del> -				
	-	‡									<del>-</del> -				
	-	‡									- -				
		<u> </u>													



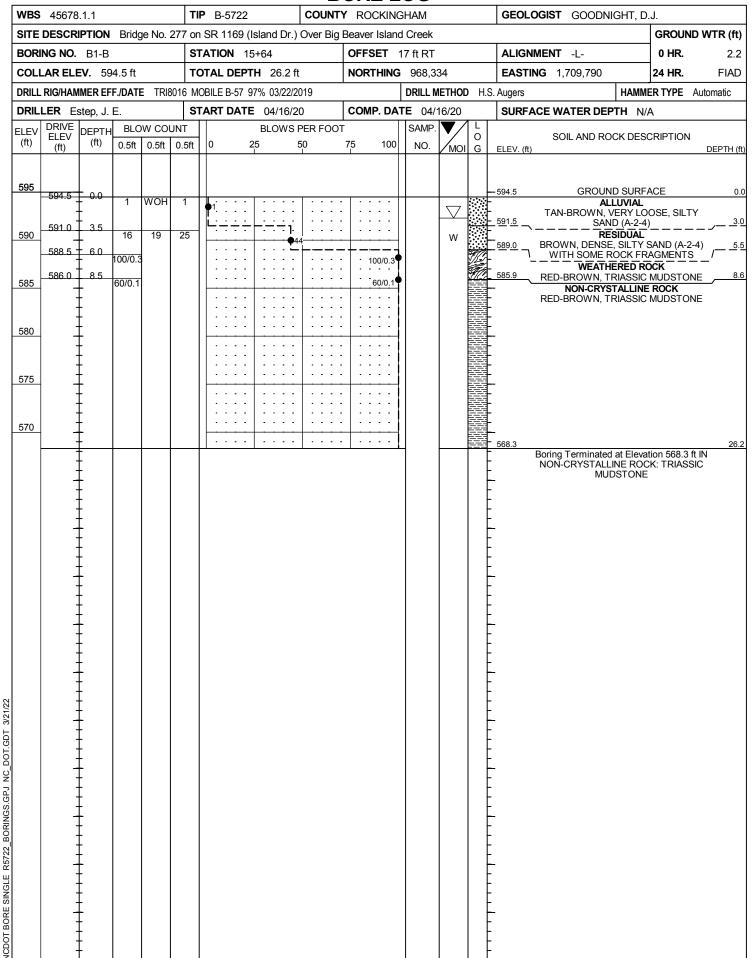




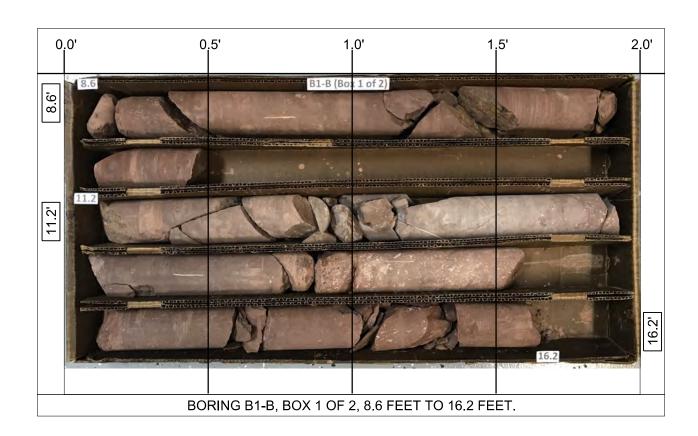


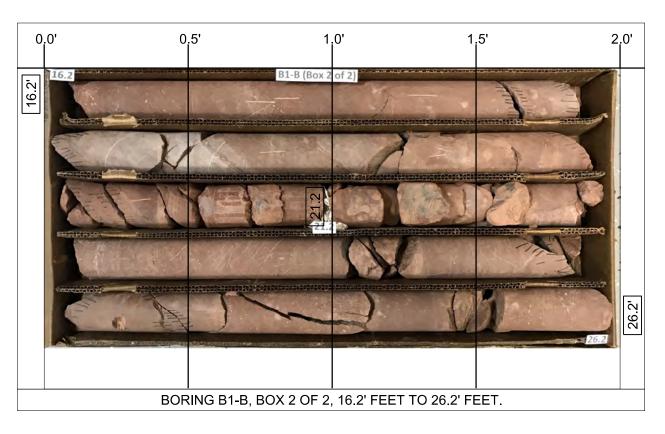
ROCK CORE PHOTOS

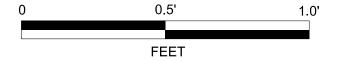
BRIDGE NO. 277 ON SR 1169 (ISLAND DRIVE) OVER BIG BEAVER ISLAND CREEK ROCKINGHAM COUNTY, NORTH CAROLINA WBS NO. 45678.1.1 | TIP NO.: B-5722 FALCON PROJECT NO.: G20003.00



									<u></u>	UI	E LOG				
	45678					B-572					OCKINGHAM	GEOLOGIST GOODN	IGHT, [		
SITE	DESCR	IPTION	Brid	lge No. 2				d Dr.)	Over E	Ť	aver Island Creek			GROUN	ID WTR (ft
BORI	NG NO.	. B1-B			STA	TION	15+64			OF	SET 17 ft RT	ALIGNMENT -L-		0 HR.	2.2
COLL	AR ELE	<b>EV</b> . 59	4.5 ft		TOT	AL DE	<b>PTH</b> 26.	.2 ft		NC	<b>PTHING</b> 968,334	<b>EASTING</b> 1,709,790		24 HR.	FIAD
DRILL	RIG/HAI	MMER E	FF./DA	TE TRI80	16 MO	BILE B-	57 97% 0	3/22/20	19		DRILL METHOD H.	S. Augers	HAMM	ER TYPE	Automatic
DRIL	LER E	step, J.	E.		STAF	RT DA	<b>TE</b> 04/1	6/20		CC	<b>IP. DATE</b> 04/16/20	SURFACE WATER DEF	TH N	/A	
CORE	E SIZE	NQ2		1			<b>N</b> 17.6 f	t	\ <u>\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \</u>						
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	REC. (ft) %	JN RQD (ft) %	SAMP. NO.	STR REC. (ft) %	RQD (ft) %	L O G	ELEV. (ft)	DESCRIPTION AND REMARK	S		DEPTH (
585.9 585	585.9	8.6	2.6	2:27/0.6	(2.2)	(1.7)		(16 E)	(10.7)		.585.9	Begin Coring @ 8.6 ft NON-CRYSTALLINE ROCI	,		8
	583.3	11.2	5.0	2:37/0.6 4:38/1.0 5:10/1.0 3:53/1.0	85% (4.9)	65% (2.9)		94%	61%		RED-BROWN, MODERATELY HAI	SLIGHTLY TO VERY SLIGHT RD TO HARD, MODERATELY D, TRIASSIC MUDSTONE (ST	LY WEAT	TO CLOSE	
580	578.3 -	16.2		4:00/1.0 4:18/1.0 5:51/1.0 5:13/1.0	98%	58%				藍	•	,		,	
575	-	10.2	5.0	5:05/1.0 5:35/1.0 4:17/1.0	(4.6) 92%	(3.6) 72%									
0.0	573.3	21.2	5.0	5:03/1.0 10:32/1.0 7:29/1.0	(4.8)	(2.5)				蓋	•				
570	-	-		4:14/1.0 4:22/1.0 5:26/1.0	96%	50%									
	568.3	26.2		4:33/1.0							568.3 Boring Terminated	at Elevation 568.3 ft IN NON-	CRYSTAI	LINE ROO	26 CK:



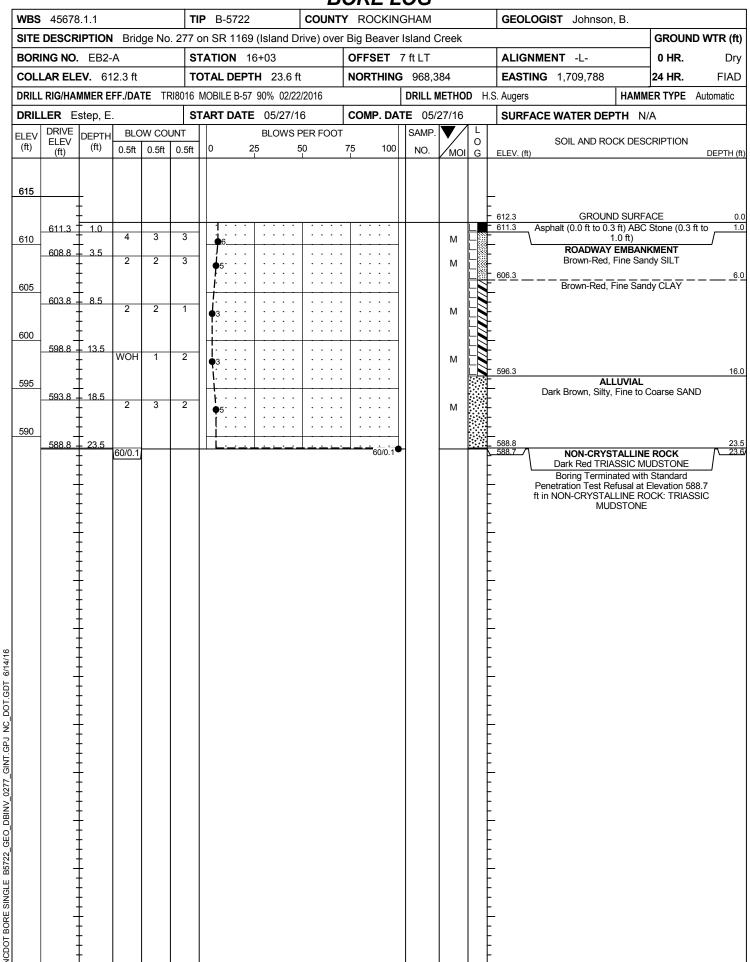


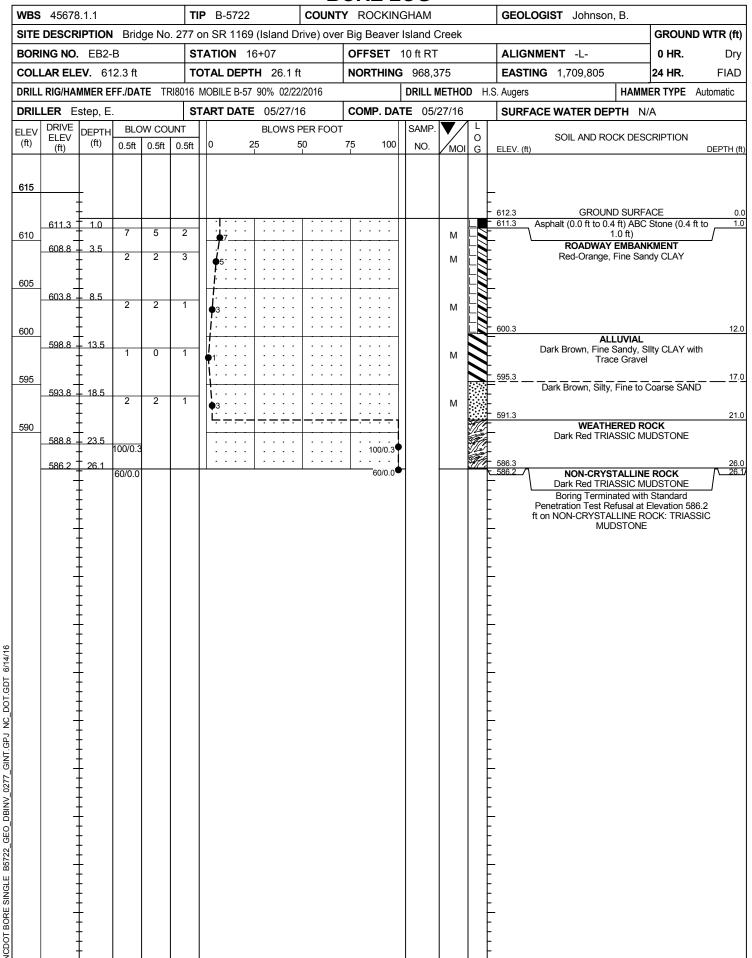


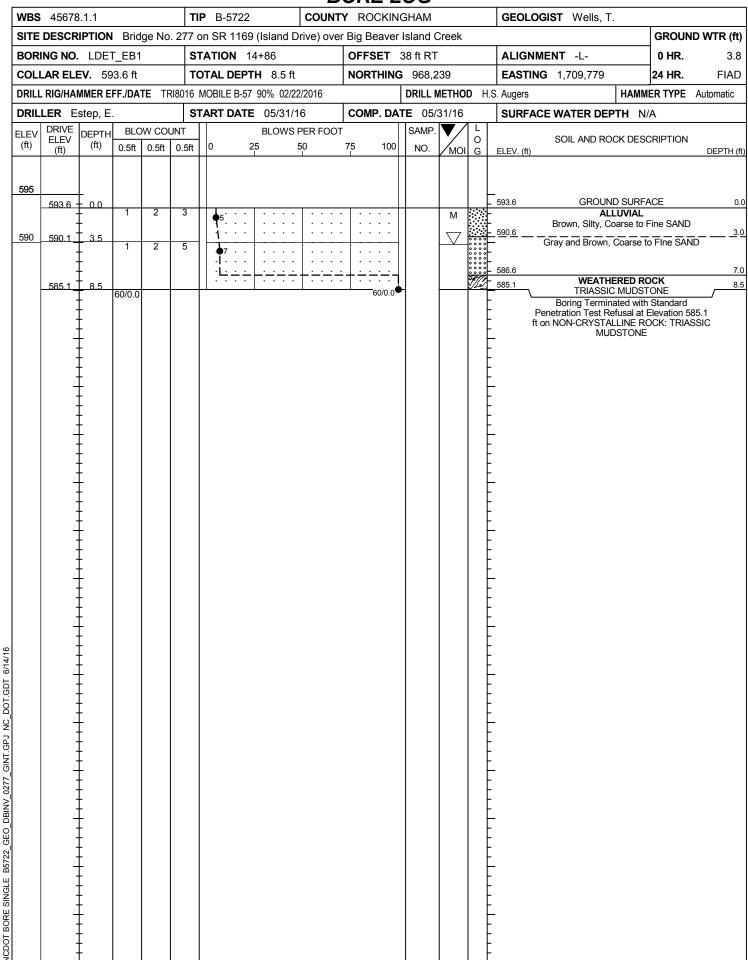


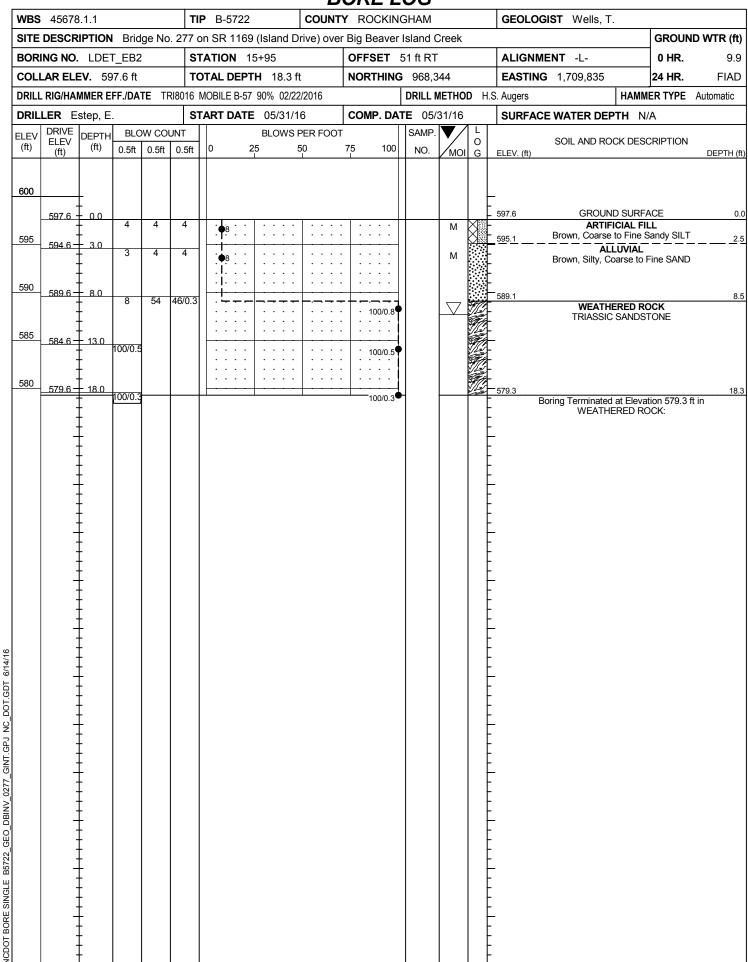
ROCK CORE PHOTOS

BRIDGE NO. 277 ON SR 1169 (ISLAND DRIVE) OVER BIG BEAVER ISLAND CREEK ROCKINGHAM COUNTY, NORTH CAROLINA WBS NO. 45678.1.1 | TIP NO.: B-5722 FALCON PROJECT NO.: G20003.00

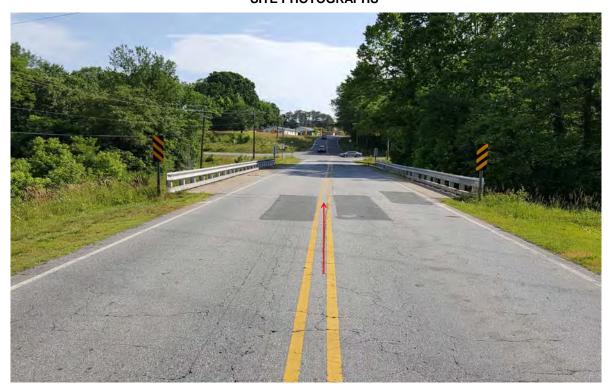








SHEET 16 45678.1.1 (B-5722) Bridge No. 277 on SR 1169 (Island Drive) over Big Beaver Island Creek SITE PHOTOGRAPHS



View Looking Upstation from End Bent No. 1



Profile of Bridge Looking West from End Bent No. 2